

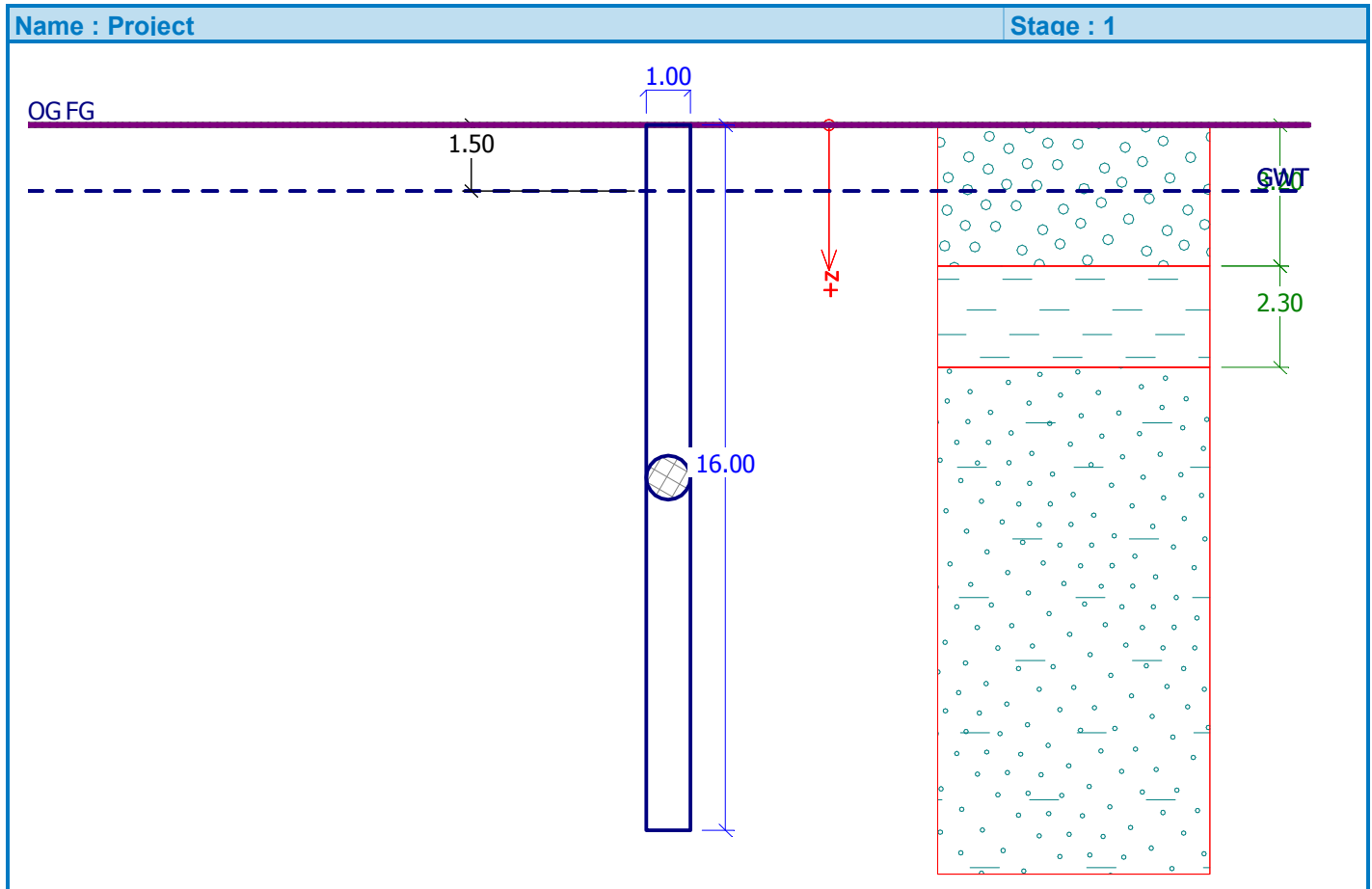


## Pile Bearing Capacity Analysis / Verification

### Input data

#### Project

**Task** : PROJECT: "NEW STEAM BOILER U-5190 in Aspropyrgos Industrial Complex"  
**Part** : A-1  
**Descript.** : The objective of this Analysis is the Pile allowable bearing Capacity Analysis % Calculations for the construction of the New Steam Boiler U-5190, in Aspropyrgos Industrial Complex.  
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**Customer** : H.P. S.A.  
**Date** : 09-Oct-13



### Basic soil parameters

| No. | Name  | Pattern | $\phi_{ef}$ [°] | $C_{ef}$ [kPa] | $\gamma$ [kN/m <sup>3</sup> ] | $\gamma_{su}$ [kN/m <sup>3</sup> ] |
|-----|---|---------|-----------------|----------------|-------------------------------|------------------------------------|
| 1   | Poorly graded gravel (GP), medium dense           |         | 35.50           | 0.00           | 19.00                         | 9.50                               |
| 2   | High plasticity clay (CH,CV,CE), consistency soft |         | 27.00           | 0.00           | 18.00                         | 9.00                               |
| 3   | Clayey sand (SC)                                  |         | 42.00           | 0.00           | 21.00                         | 11.50                              |

All soils are considered as cohesionless for at rest pressure analysis.



| No. | Name  | Pattern | $E_{oed}$<br>[MPa] | $E_{def}$<br>[MPa] | $\gamma_{sat}$<br>[kN/m <sup>3</sup> ] | $\gamma_s$<br>[kN/m <sup>3</sup> ] | n<br>[-] |
|-----|---|---------|--------------------|--------------------|--|------------------------------------|----------|
| 1   | Poorly graded gravel (GP), medium dense           |         | 161.00             | -                  | 19.50                                  | -                                  | -        |
| 2   | High plasticity clay (CH,CV,CE), consistency soft |         | 8.00               | -                  | 19.00                                  | -                                  | -        |
| 3   | Clayey sand (SC)                                  |         | 40.00              | -                  | 21.50                                  | -                                  | -        |

#### Parameters of soils to compute modulus of subsoil reaction

| No. | Name  | Pattern | $\beta$ |
|-----|---|---------|---------|
| 1   | Poorly graded gravel (GP), medium dense           |         | 15.00   |
| 2   | High plasticity clay (CH,CV,CE), consistency soft |         | 10.00   |
| 3   | Clayey sand (SC)                                  |         | 25.00   |

#### Soil parameters

##### Poorly graded gravel (GP), medium dense

Unit weight :  $\gamma = 19.00$  kN/m<sup>3</sup>  
 Angle of internal friction :  $\phi_{ef} = 35.50^\circ$   
 Cohesion of soil :  $c_{ef} = 0.00$  kPa  
 Poisson's ratio :  $\nu = 0.20$   
 Oedometric modulus :  $E_{oed} = 161.00$  MPa  
 Saturated unit weight :  $\gamma_{sat} = 19.50$  kN/m<sup>3</sup>  
 Angle of dispersion :  $\beta = 15.00^\circ$

##### High plasticity clay (CH,CV,CE), consistency soft

Unit weight :  $\gamma = 18.00$  kN/m<sup>3</sup>  
 Angle of internal friction :  $\phi_{ef} = 27.00^\circ$   
 Cohesion of soil :  $c_{ef} = 0.00$  kPa  
 Poisson's ratio :  $\nu = 0.42$   
 Oedometric modulus :  $E_{oed} = 8.00$  MPa  
 Saturated unit weight :  $\gamma_{sat} = 19.00$  kN/m<sup>3</sup>  
 Angle of dispersion :  $\beta = 10.00^\circ$

##### Clayey sand (SC)

Unit weight :  $\gamma = 21.00$  kN/m<sup>3</sup>  
 Angle of internal friction :  $\phi_{ef} = 42.00^\circ$   
 Cohesion of soil :  $c_{ef} = 0.00$  kPa  
 Poisson's ratio :  $\nu = 0.35$   
 Oedometric modulus :  $E_{oed} = 40.00$  MPa  
 Saturated unit weight :  $\gamma_{sat} = 21.50$  kN/m<sup>3</sup>  
 Angle of dispersion :  $\beta = 25.00^\circ$



## Geometry of structure

### Pile geometry

Pile profile: circular

### Dimensions

Diameter  $d = 1.00$  m

Length  $l = 16.00$  m

### Location

Off ground height  $h = 0.00$  m

Depth of finished grade  $h_z = 0.00$  m

### Technology

Piles with excavation of soil from a bore hole

Pile type: bored with or without clayey suspension

Heel resistance reduction = 0.50

Skin resistance reduction = 0.60

Modulus of subsoil reaction assumed constant.

## Material of structure

Analysis of concrete structures carried out according to the standard EN 1992 1-1 (EC2).

Concrete : C 40/50

Longitudinal steel : B500

## Geological profile and assigned soils

| No. | Layer [m] | Assigned soil                                     | Pattern |
|-----|-----------|---|---------|
| 1   | 3.20      | Poorly graded gravel (GP), medium dense           |         |
| 2   | 2.30      | High plasticity clay (CH,CV,CE), consistency soft |         |
| 3   | -         | Clayey sand (SC)                                  |         |

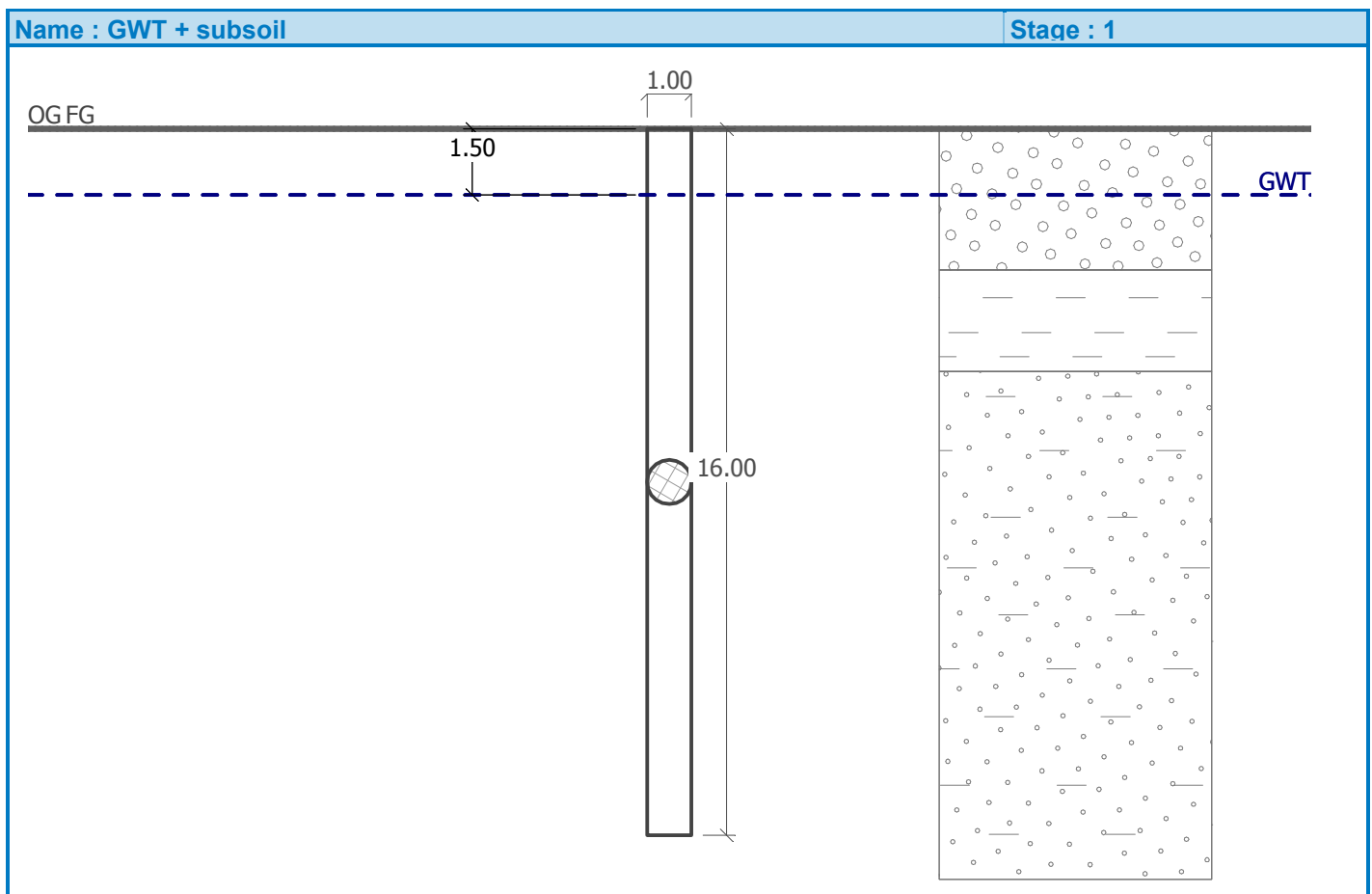
## Assumed Loads

| No. | Load |        | Name       | Type   | N [kN]  | $M_x$ [kNm] | $M_y$ [kNm] | $H_x$ [kN] | $H_y$ [kN] |
|-----|------|--------|------------|--------|---------|-------------|-------------|------------|------------|
|     | new  | change |            |        |         |             |             |            |            |
| 1   | YES  |        | Load No. 1 | Design | 2842.00 | 500.00      | 250.00      | 100.00     | 50.00      |



### Ground water table

The ground water table is at a depth of 1.50 m below the original terrain elevation.



### Analysis settings

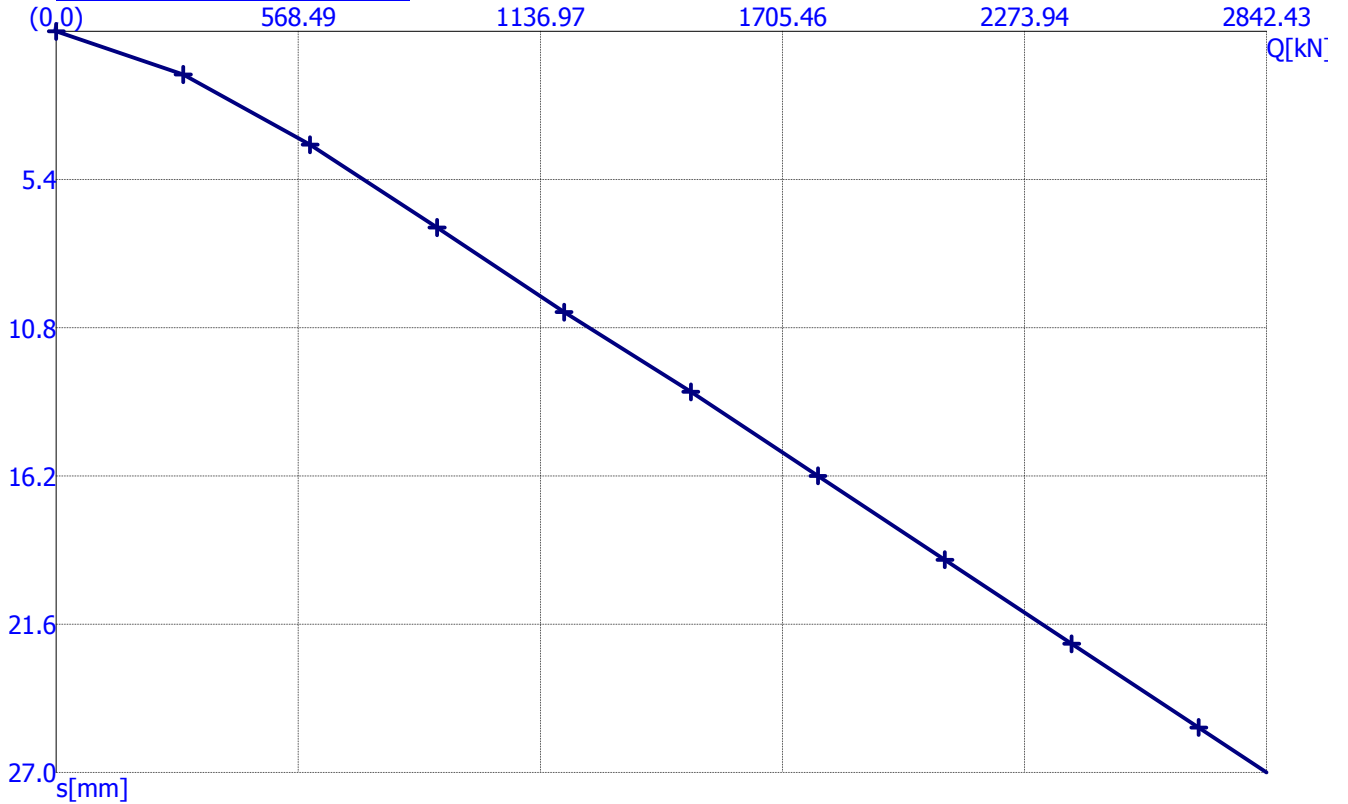
Analysis carried out without reduction of input data.



Name : Vert. cap. (springs)

Stage : 1: Verification : 1

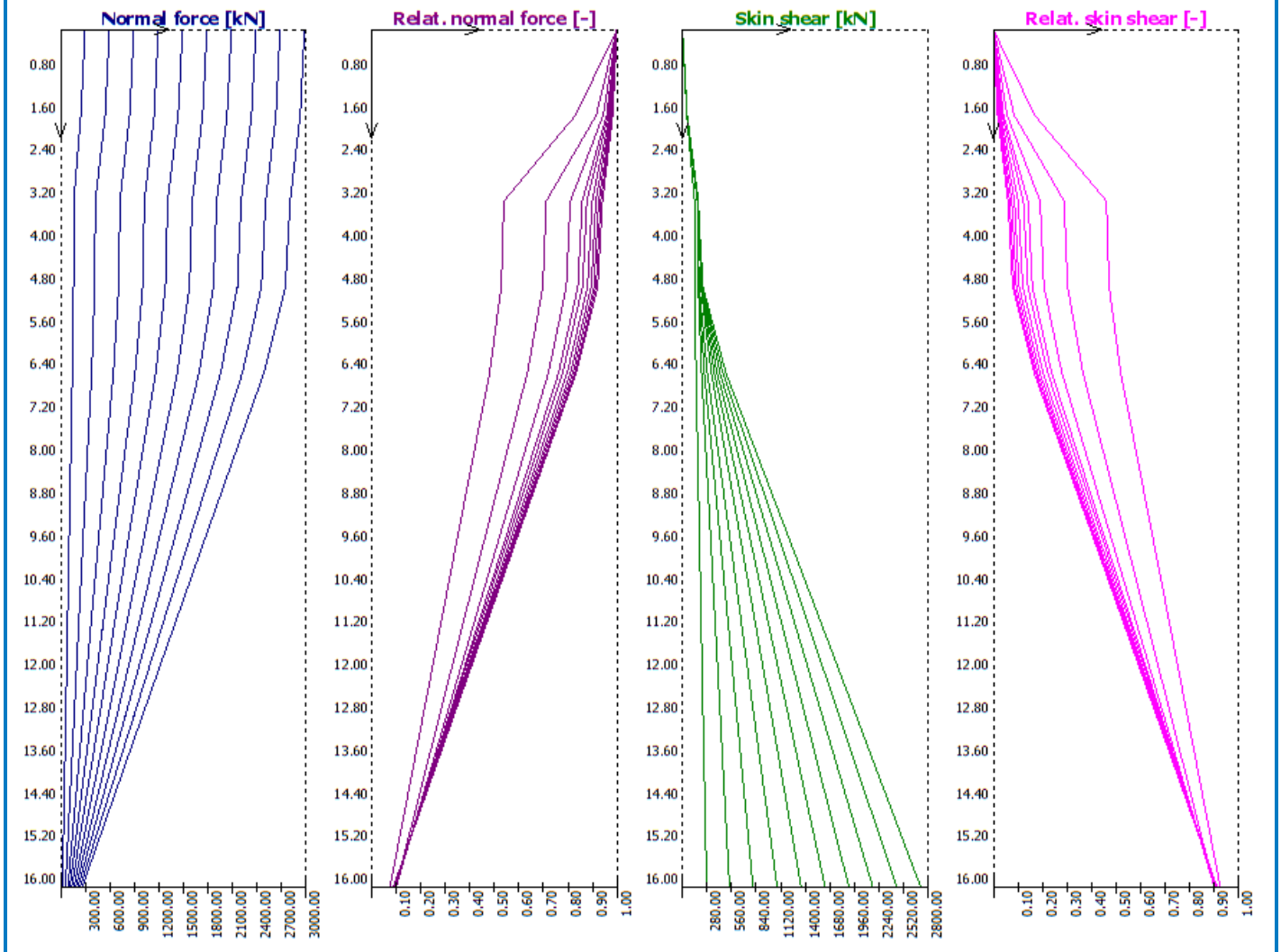
**Ultimate load transfer curve**





Name : Vert. cap. (springs)

Stage : 1: Verification : 1



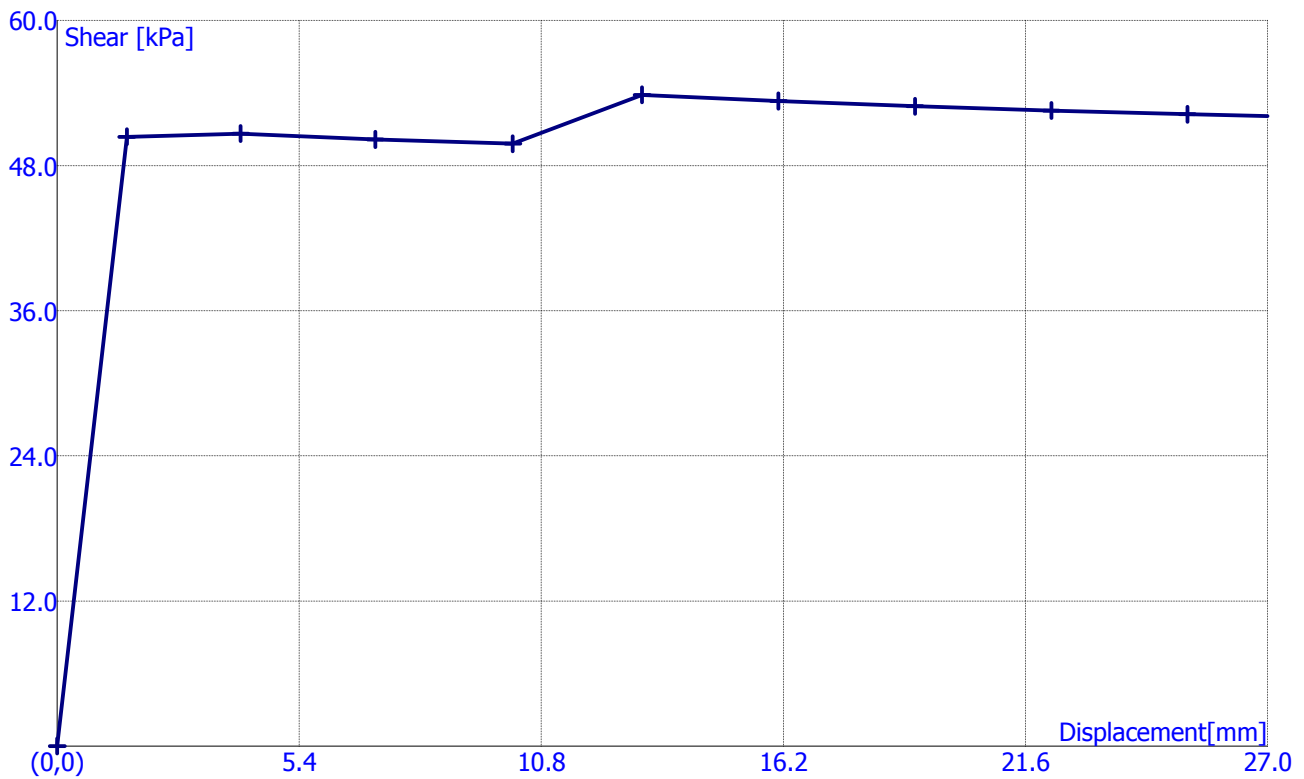


## Verification No. 1

Name : Vert. cap. (springs)

Stage : 1: Verification : 1

### Shear - deformation dependence (at a depth of 0.00m)

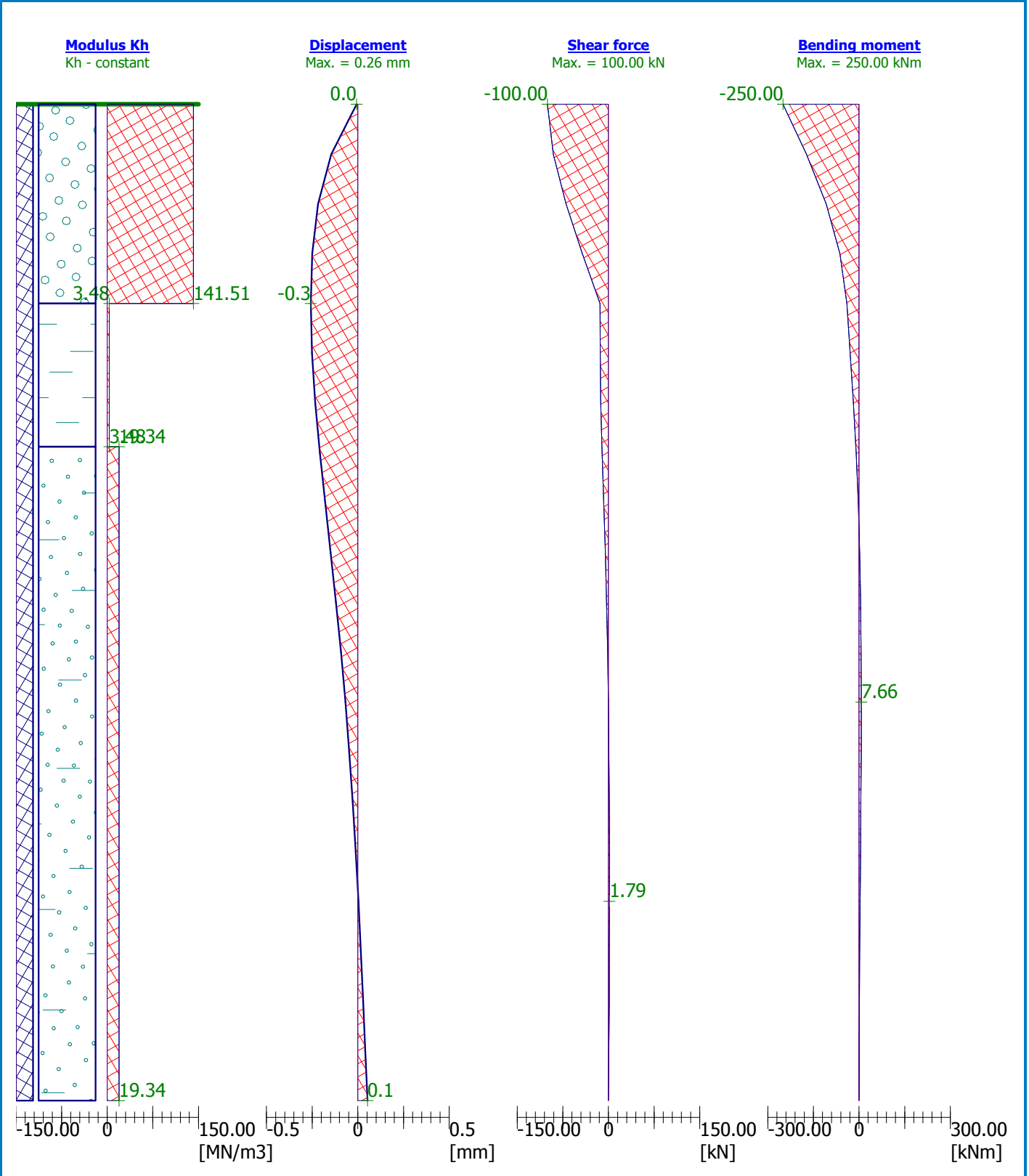




**Verification No. 1**

Name : Horizontal cap.

Stage : 1: Verification : 1

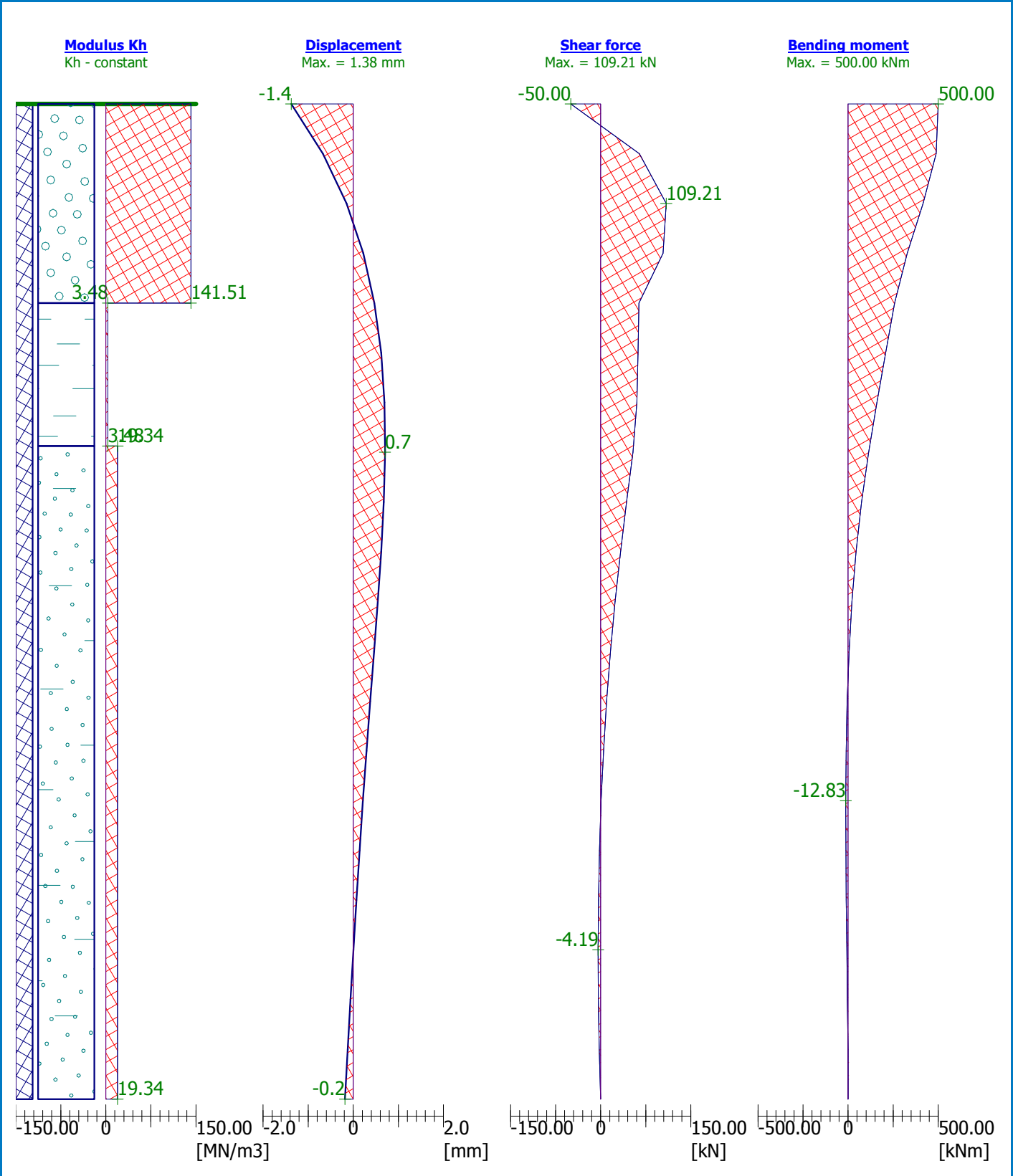






**Verification No. 1**

Name : Horizontal cap. Stage : 1: Verification : 1

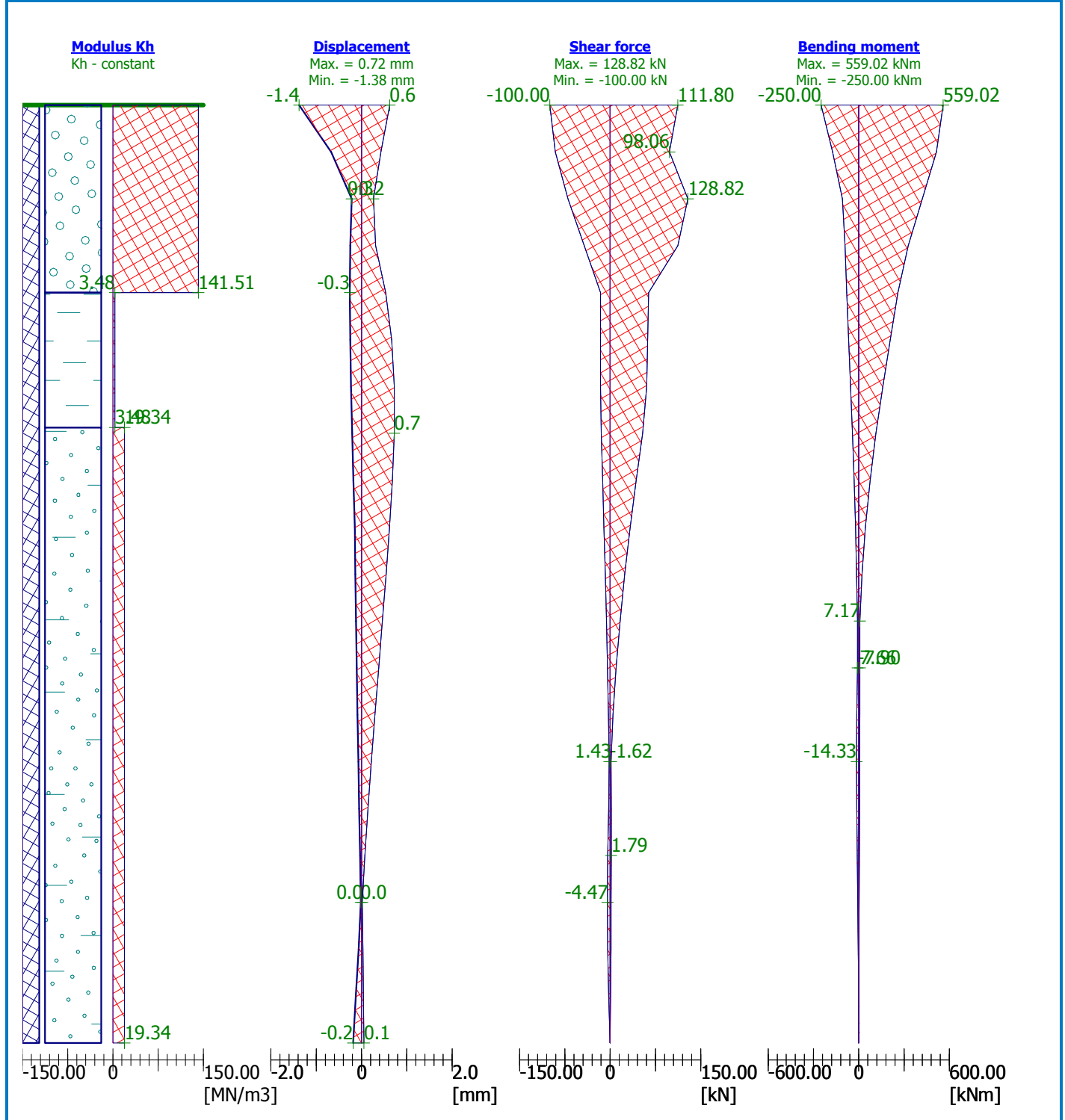




### Verification No. 1

Name : Horizontal cap.

Stage : 1; Verification : 1





**Maximum internal force and deformation :**

Max. pile displacement = 1.4 mm  
Max. shear force = 128.82 kN  
Maximum moment = 559.02 kNm

**Dimensioning of reinforcement:**

Reinforcement - 6 pc bars 30.0 mm; Nominal covering 40.0 mm

Reinforcement ratio  $\rho = 0.270 \% > 0.182 \% = \rho_{min}$

Load :  $N_{Ed} = -2842.00$  kN (compression) ;  $M_{Ed} = 559.02$  kNm

Bearing capacity :  $N_{Rd} = -14135.36$  kN;  $M_{Rd} = 1791.43$  kNm

**Designed pile reinforcement is SATISFACTORY.**

Athens, 09 October - 2013

On behalf of and for Geodomisi Ltd.

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